C2, C3W, C4, C5, C6 Flood Protection Level of Service for Current and Future SLR





September 12, 2023

Presentation Outline

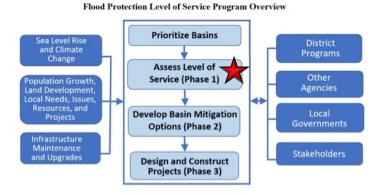
- 1. FPLOS Overview
- 2. Model Selection
- 3. Study Area and Model Domain
- 4. Model Setup
- 5. Current/Future Conditions Setup
- 6. FPLOS Metrics
- 7. Current/Future Results
- 8. Preliminary Mitigation Strategies

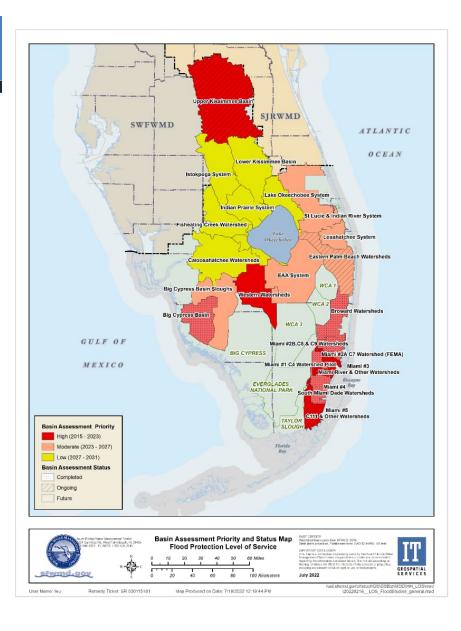




FPLOS Program Overview

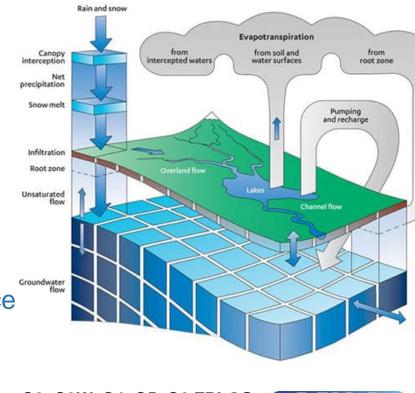
- To fulfill the need of long-term flood protection for basins throughout the 16-county region, a flood protection level of service (FPLOS) program has been established.
- Mission:
 - Identify and prioritize long-term infrastructure improvement needs
 - Develop an implementation strategy
- Goal: to assure that each basin can maintain its designated FPLOS, in a technical and cost-effective manner, in response to population growth, land development, sea level rise and climate conditions change.





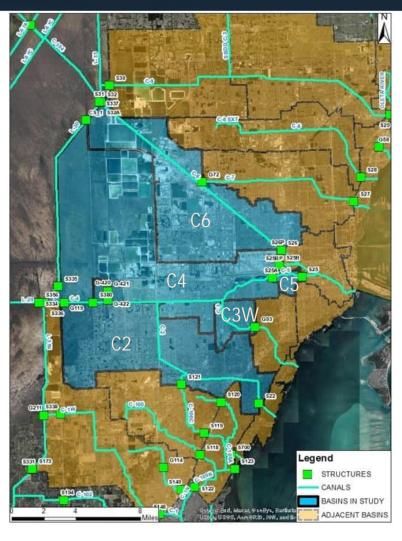
Model Selection

- 1. HEC/HMS/HEC-RAS/HEC-RAS2D
- 2. ICPR4
- 3. EPA SWMM5/XP-SWMM
- 4. MIKE SHE/MIKE HYDRO River
 - Hydrology → two-dimensional, physicallybased representation of the land surface
 - Hydraulics → allows for control structure operations
 - Groundwater → Interconnectivity with surface water, while accommodating multiple groundwater layers





Study Area



Watersheds Evaluated

- C2 Snapper Creek
 - Outfall: S22
- C3W Coral Gables Canal
 - Outfall: G93
- C4 Tamiami Canal
 - Outfall: S25B

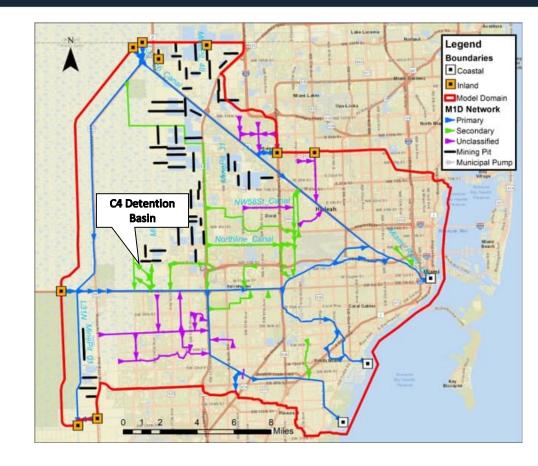
- C5 Comfort Canal
 - Outfall: S25
- C6 Miami Canal
 - Outfall: S26

All watersheds were combined in a single model



Model Setup

- Model Domain
 - From WCA/ENP to coast
 - From C7 to C2
- Model Resolution
 - 250-ft grid cell
- Calibration (5/10/20-6/1/20)
 - Dry antecedent conditions
 - Recent storm with available data
- Validation (8/20/17-9/22/17) Irma
 - Very wet antecedent conditions
 - Recent high intensity storm





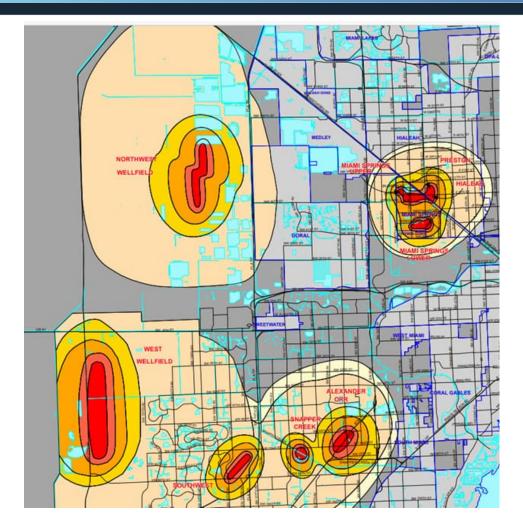
Model Setup – Spatial Data



- Topography (SFWMD LiDAR)
- Canal geometry (Existing models)
- Landuse (FDEP / FLUCCS)
 - ET Parameters
 - Overland Roughness
 - Percent Impervious
- Detention Storage (Landuse and ERPs)
- Soils Map (NRCS)
- Groundwater (From SFWMD / ECSM)



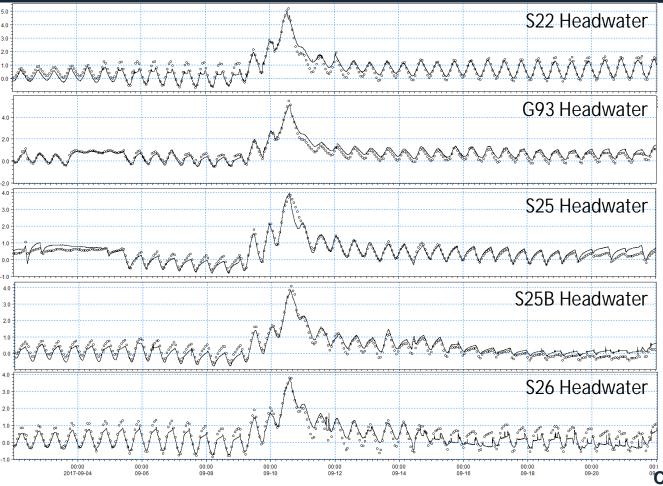
Model Setup – Temporal Data



- Rainfall NEXRAD @ 15-min
- ET Measured @ S331W
- GW Pumping Measured by MD-WASD
- GW Boundaries Measured by USGS
- Gate Operations DBHYDRO
- Tidal Data DBHYDRO
- Canal Stages DBHYDRO



Model Calibration and Validation Results



- Good representation of stages throughout each watershed
- Particularly good match-up for the Irma simulation, due to wet antecedent conditions.

[®]C2, C3W, C4, C5, C6 FPLOS for Current and Future SLR



Model Calibration and Validation Results

		Calib	ration Stag	es (5/11/2	020 - 5/30/	2020)
Watershed	Station	ME (ft)	MAE (ft)	RMSE (ft)	R	NS
	C2SW1	0.20	0.32	0.42	0.98	0.56
C2	C2SW2	0.24	0.36	0.46	0.75	-0.95
	S22_H	0.16	0.46	0.54	0.39	-0.41
C3W	G93_H	0.05	0.33	0.39	0.74	0.19
	C4.CORAL	0.32	0.42	0.49	0.95	0.24
	S25B_H	0.14	0.25	0.34	0.78	0.50
C4	\$336_T	-0.03	0.26	0.32	0.97	0.84
- 4	S380_H	0.59	0.62	1.02	0.08	-3.29
	S380_T	0.82	0.85	1.21	0.57	-1.80
	T5W	0.22	0.36	0.44	0.99	0.72
C5	S25_H	-0.26	0.42	0.48	0.82	0.36
C6	S26_H	-0.06	0.23	0.28	0.86	0.72
	\$31_T	0.21	0.24	0.28	0.97	0.82

		Vali	dation Sta	ges (9/1/20	17 - 9/30/2	.017)
Watershed	Station	ME (ft)	MAE (ft)	RMSE (ft)	R	NS
	C2SW1	0.19	0.24	0.30	0.97	0.84
C2	C2SW2	0.18	0.19	0.24	0.98	0.90
	S22_H	0.05	0.18	0.24	0.96	0.92
C3W	G93_H	-0.18	0.25	0.31	0.96	0.88
	C4.CORAL	0.01	0.30	0.45	0.70	0.34
	S25B_H	-0.11	0.29	0.35	0.91	0.81
C4	S336_T	0.11	0.34	0.40	0.51	0.20
- 04	S380_H	1.55	1.55	1.61	0.82	-10.31
	S380_T	1.60	1.60	1.73	0.59	-18.41
	T5W	0.33	0.35	0.43	0.96	0.48
C5	S25_H	-0.09	0.21	0.43	0.84	0.61
C6	S26_H	0.04	0.23	0.29	0.29 0.90	
	S31_T	0.30	0.36	0.42	0.92	0.69

- 85% of stage stations within the critical watersheds meet the MAE criteria of 0.5ft for both Calibration and Validation runs
- Simulated peak flows match measured within +/- 20% for all coastal structures during the Irma event

	Calibration Peak Flows								
Station	Measured (cfs)	Simulated (cfs)	% Difference						
S22	1533.1	1654.5	7.9%						
G93	303.3	430.9	42.1%						
S25	299.7	412.0	37.5%						
S25B	2007.1	1891.5	-5.8%						
S26	1426.6	1668.4	16.9%						

	Validation Peak Flows							
Station	Measured (cfs)	Simulated (cfs)	% Difference					
S22	1690.8	1632.0	-3.5%					
G93	362.9	362.5	-0.1%					
S25	190.9	190.3	-0.3%					
S25B	1925.7	2077.3	7.9%					
S26	1627.3	1323.8	-18.6%					



Design Storm Simulations - Current and Future Conditions

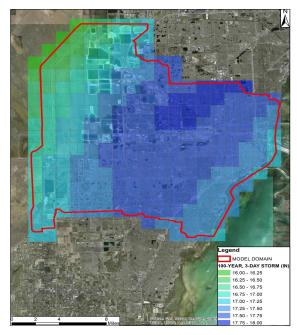
Design Storm Setup

- Structure Operations
 - District's Structure Atlas,
 - C4 Watershed Operations Plan
 - Water managers

	1	C 1 1					
See 1 1 1		Structure					
Branch Name	Name	Туре	Logic Language				
			Unchanged if TSLGLC<6min, Else Closed if dH<0.1, Else Fully				
Coral Gables		Sluice,	Open if Hups>0.76 ft-NAVD AND dH>0.3ft, Else Closed if				
Canal	G93_G1	Formula	Hups<-0.54 ft-NAVD, Otherwise Unchanged				
			Unchanged if TSLGLC<6min, Else Closed if dH<0.1, Else Fully				
Coral Gables		Sluice,	Open if Hups>0.76 ft-NAVD AND dH>0.3ft, Else Closed if				
Canal	G93 G2	Formula	Hups<-0.54 ft-NAVD, Otherwise Unchanged				
			Unchanged if TSLGLC<6min, Else Closed if dH<0.1ft, Else Fully				
Snapper Creek		Sluice,	Open if Hups > 0.96 ft-NAVD AND dH>0.3ft, Else Closed if Hups				
Canal	S22_G1	Formula	< -0.04 ft-NAVD, Otherwise Unchanged				
		104.57	Unchanged if TSLGLC<6min, Else Closed if dH<0.1ft, Else Fully				
Snapper Creek		Sluice,	Open if Hups > 0.96 ft-NAVD AND dH>0.3ft, Else Closed if Hups				
Canal	S22_G2	Formula	< -0.04 ft-NAVD, Otherwise Unchanged				
			Unchanged if TSLGLC<6min, Else Closed if dh<0.1, Else Fully				
Comfort Canal			Open if Hups>-0.345 ft-NAVD, Else Closed if Hups<-0.745 ft-				
Southfork	S25	Underflow	NAVD, Else Fully Open if dh>0.3ft, Otherwise Unchanged				
Comfort_Canal							
Southfork	S25A_G1	Underflow	Closed				
			Unchanged if TSLGLC<6min, Else Closed if dh<0.1ft, Else Fully				
		Sluice,	Open if Hups>0.451 ft-NAVD, Else Closed if Hups<-0.549 ft-				
C4_Canal	S25B_G1	Formula	NAVD, Else Fully Open if dH>0.3ft, Otherwise Unchanged				
H = head, dH = o	delta or differe	ence in head be	tween upstream and downstream of the gate,				
Hups = head unstream of structure, TSLGLC - Time Since Last Gate Level Change							

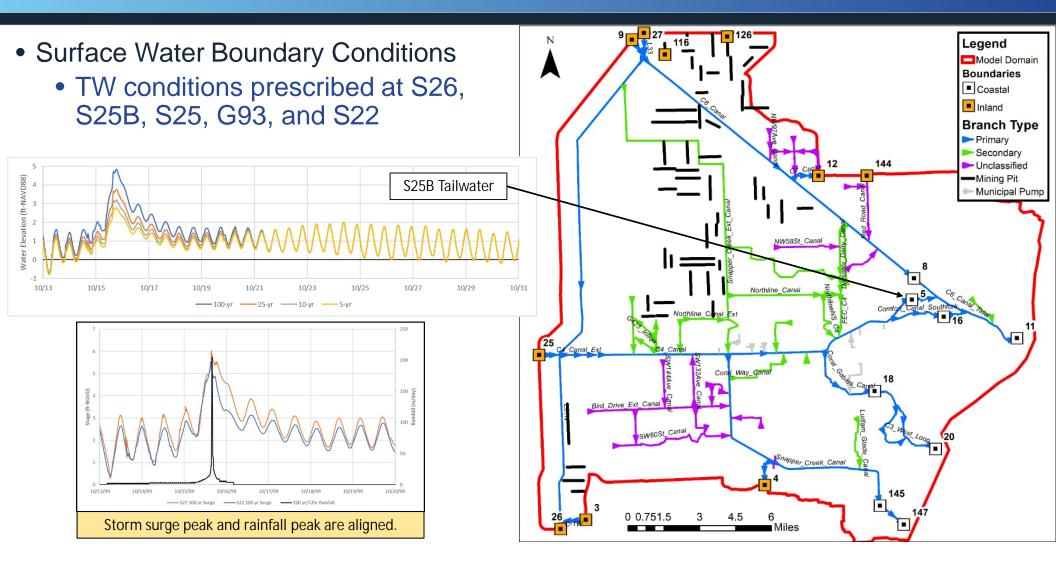
Hups = head upstream of structure, 1SLGLC Time Since Last Gate

- Rainfall Events
 - 5/10/25/100-year, 3-day storms
 - Spatially distributed using NEXRAD grid

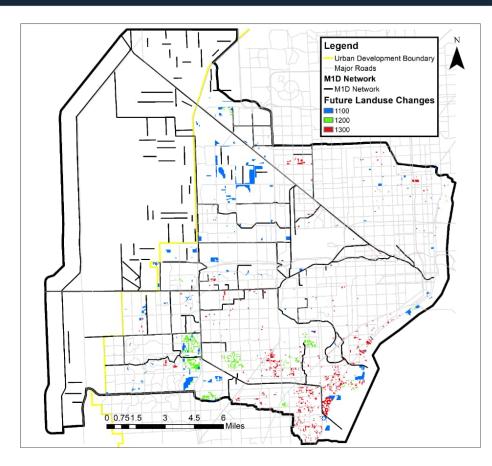




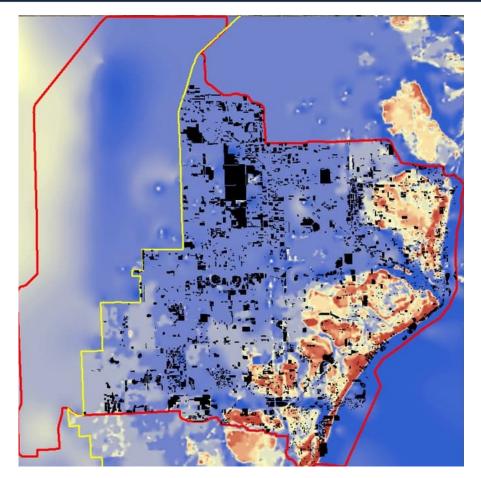
Design Storm Setup



- Land use
 - Increase LU classification
 - Open/Vacant/Ag to Low Density
 - Low Density to Medium Density
 - Medium Density to High Density
- Overland Parameters
 - Adjusted for more development
 - Manning's n
 - Detention storage
 - Runoff coefficient



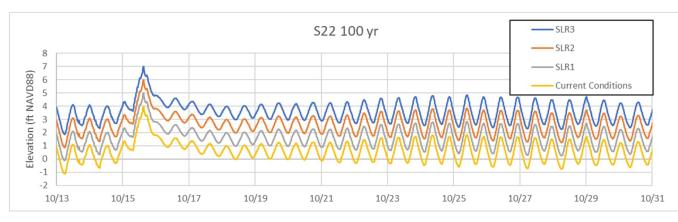




- Topography
 - Raised elevation for 90% of each vacant and old parcel (MDC Flood Criteria)
 - Reduced elevation for 10% of each vacant and old parcel to account for onsite storage



- Surface Water
 - Increased initial conditions and boundary conditions to account for SLR (+1, +2, +3 ft)
 - Added "planned canals" from MDC with cross-sections matching TOB criteria

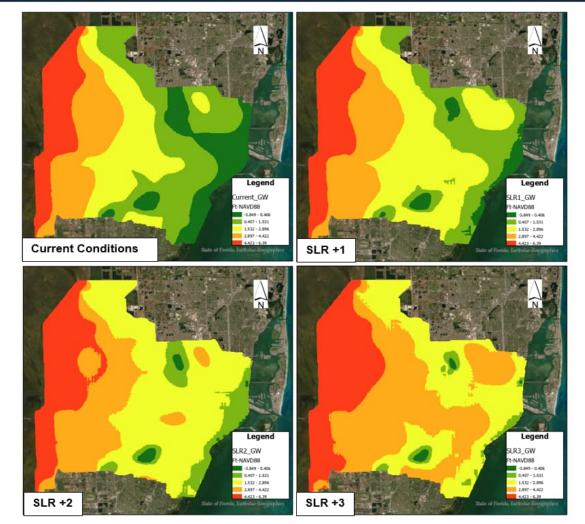






Groundwater

- Used MDC's future GW projections for SLR +1 ft (May 2040)
- Projected future GW for SLR +2 and +3 ft
- Increased water supply withdrawals based on MD-WASD estimates (+44%)



Modeling Results and Mitigation Strategies

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C2, C3W, C4, C5, C6 FPLOS for Current and Future SLR

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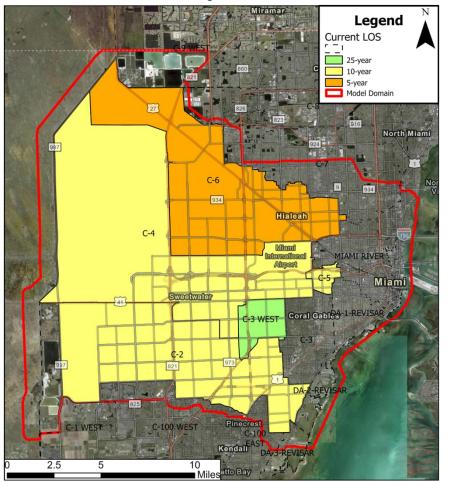
FPLOS Metrics

- SFWMD developed 6 FPLOS performance metrics (PMs)
- Comparison tool for changes in flood protection as a result of SLR

Performance Metric	Description
PM #1	Maximum stage in primary canals
PM #2	Maximum discharge capacity through the primary canals
PM #3	Tidal structure flow performance
PM #4	Peak storm runoff – maximum conveyance capacity of the watershed
PM #5	Frequency of flooding – stage-based LOS for sub-watersheds
PM #6	Duration of flooding



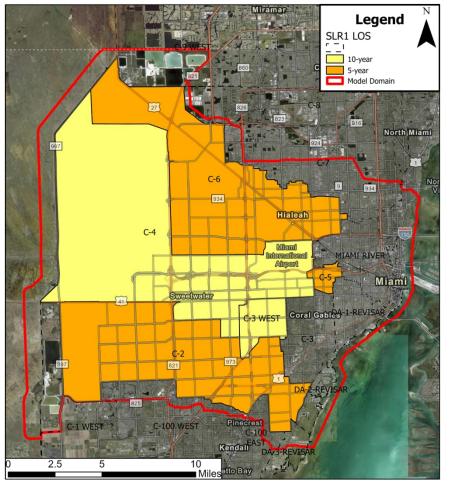
Overall LOS Rating for Each Watershed



CONDITION	PM	C2	C3W	C4	C5	C6	
	PM #1	10-year	25-year	25-year	25-year	25-year	
Cumment	PM #5	25-year	25-year	10-year	10-year	5-year	
Current Conditions	PM #6	25-year	25-year	10-year	25-year	<5-year	
	Overall LOS	10-year	25-year	10-year	10-year	5-year	
	PM #1	5-year	10-year	10-year	10-year	10-year	
Future	PM #5	25-year	25-year	10-year	5-year	5-year	
Conditions	PM #6	25-year	10-year	10-year	10-year	<5-year	
SLR +1 foot	Overall LOS	5-year	10-year	10-year	5-year	5-year	
	PM #1	<5-year	5-year	5-year	<5-year	<5-year	
Future	PM #5	10-year	10-year	5-year	5-year	<5-year	
Conditions	PM #6	10-year	10-year	5-year	5-year	<5-year	
SLR +2 feet	Overall LOS	<5-year	5-Year	5-year	<5-year	<5-year	
	PM #1	<5-year	<5-year	<5-year	<5-year	<5-year	
Future	PM #5	10-year	10-year	<5-year	<5-year	<5-year	
Conditions	PM #6	10-year	5-year	<5-year	<5-year	<5-year	
SLR +3 feet	Overall LOS	<5-year	<5-year	<5-year	<5-year	<5-year	



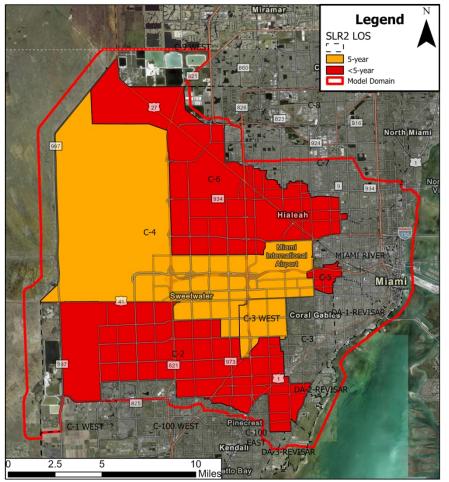
Overall LOS Rating for Each Watershed



CONDITION	PM	C2	C3W	C4	C5	C6
	PM #1	10-year	25-year	25-year	25-year	25-year
Current	PM #5	25-year	25-year	10-year	10-year	5-year
Current Conditions	PM #6	25-year	25-year	10-year	25-year	<5-year
Containiono	Overall LOS	10-year	25-year	10-year	10-year	5-year
	PM #1	5-year	10-year	10-year	10-year	10-year
Future	PM #5	25-year	25-year	10-year	5-year	5-year
Conditions	PM #6	25-year	10-year	10-year	10-year	<5-year
SLR +1 foot	Overall LOS	5-year	10-year	10-year	5-year	5-year
	PM #1	<5-year	5-year	5-year	<5-year	<5-year
Future	PM #5	10-year	10-year	5-year	5-year	<5-year
Conditions	PM #6	10-year	10-year	5-year	5-year	<5-year
SLR +2 feet	Overall LOS	<5-year	5-Year	5-year	<5-year	<5-year
	PM #1	<5-year	<5-year	<5-year	<5-year	<5-year
Future	PM #5	10-year	10-year	<5-year	<5-year	<5-year
Conditions	PM #6	10-year	5-year	<5-year	<5-year	<5-year
SLR +3 feet	Overall LOS	<5-year	<5-year	<5-year	<5-year	<5-year



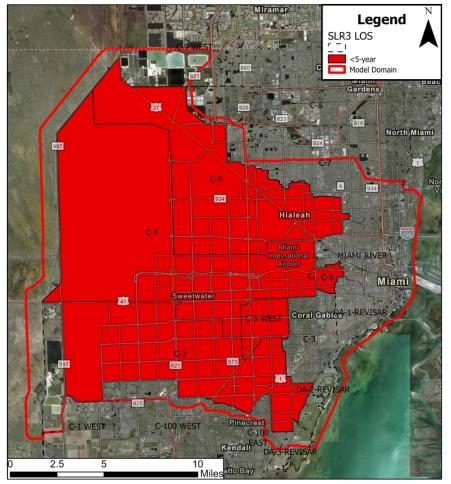
Overall LOS Rating for Each Watershed



CONDITION	РМ	C2	C3W	C4	C5	C6
	PM #1	10-year	25-year	25-year	25-year	25-year
Current	PM #5	25-year	25-year	10-year	10-year	5-year
Conditions	PM #6	25-year	25-year	10-year	25-year	<5-year
	Overall LOS	10-year	25-year	10-year	10-year	5-year
	PM #1	5-year	10-year	10-year	10-year	10-year
Future	PM #5	25-year	25-year	10-year	5-year	5-year
Conditions	PM #6	25-year	10-year	10-year	10-year	<5-year
SLR +1 foot	Overall LOS	5-year	10-year	10-year	5-year	5-year
	PM #1	<5-year	5-year	5-year	<5-year	<5-year
Future	PM #5	10-year	10-year	5-year	5-year	<5-year
Conditions	PM #6	10-year	10-year	5-year	5-year	<5-year
SLR +2 feet	Overall LOS	<5-year	5-Year	5-year	<5-year	<5-year
	PM #1	<5-year	<5-year	<5-year	<5-year	<5-year
Future	PM #5	10-year	10-year	<5-year	<5-year	<5-year
Conditions	PM #6	10-year	5-year	<5-year	<5-year	<5-year
SLR +3 feet	Overall LOS	<5-year	<5-year	<5-year	<5-year	<5-year

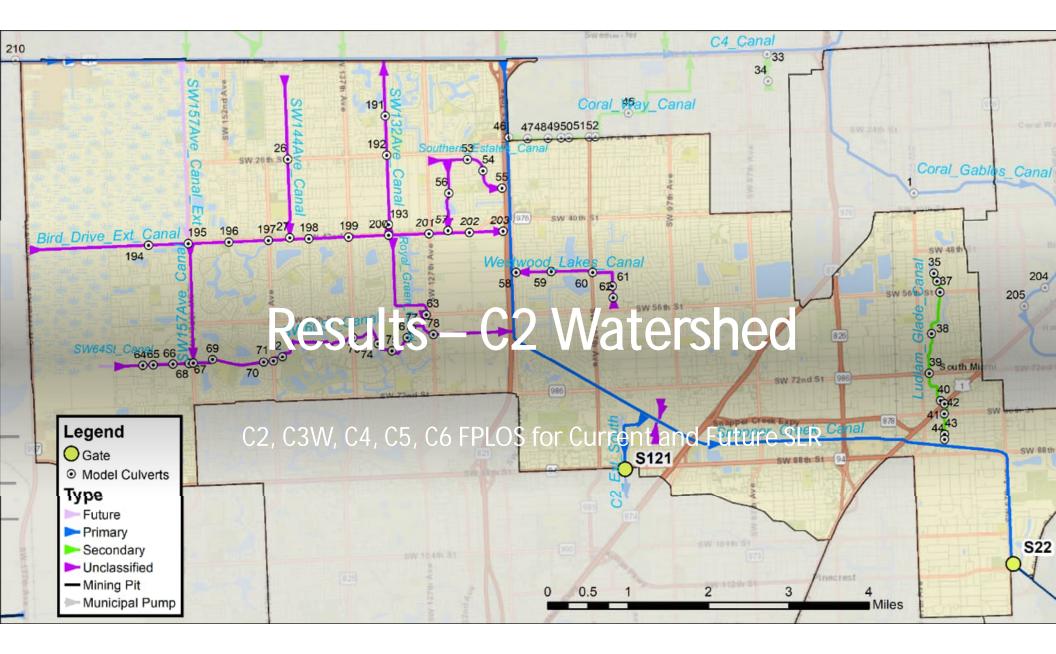


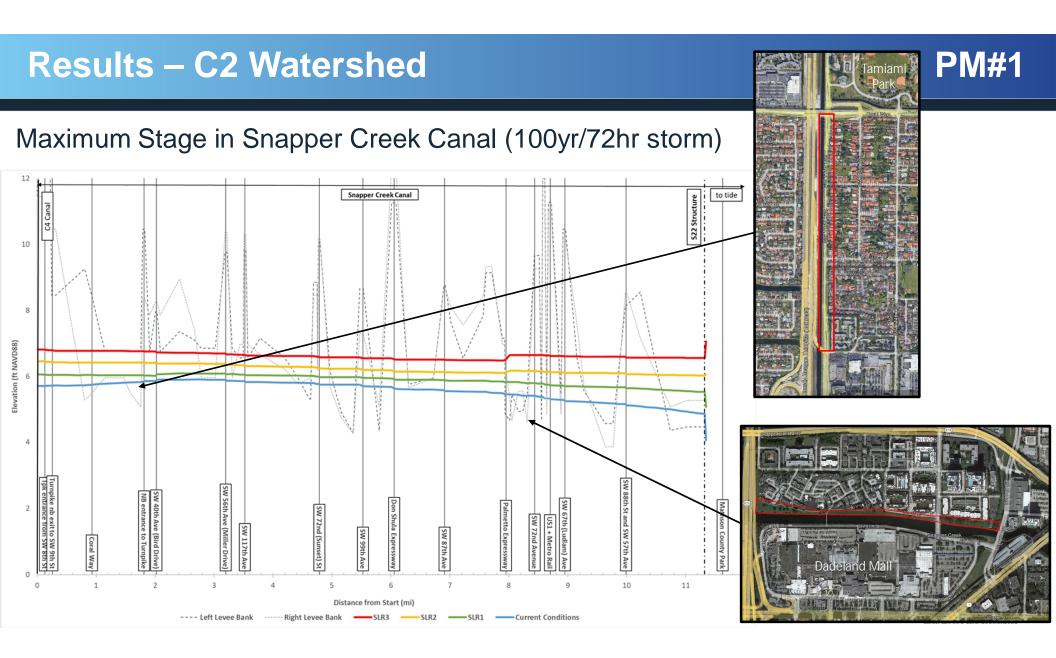
Overall LOS Rating for Each Watershed



CONDITION	РМ	C2	C3W	C4	C5	C6
	PM #1	10-year	25-year	25-year	25-year	25-year
Current	PM #5	25-year	25-year	10-year	10-year	5-year
Current Conditions	PM #6	25-year	25-year	10-year	25-year	<5-year
	Overall LOS	10-year	25-year	10-year	10-year	5-year
	PM #1	5-year	10-year	10-year	10-year	10-year
Future	PM #5	25-year	25-year	10-year	5-year	5-year
Conditions	PM #6	25-year	10-year	10-year	10-year	<5-year
SLR +1 foot	Overall LOS	5-year	10-year	10-year	5-year	5-year
	PM #1	<5-year	5-year	5-year	<5-year	<5-year
Future	PM #5	10-year	10-year	5-year	5-year	<5-year
Conditions	PM #6	10-year	10-year	5-year	5-year	<5-year
SLR +2 feet	Overall LOS	<5-year	5-Year	5-year	<5-year	<5-year
	PM #1	<5-year	<5-year	<5-year	<5-year	<5-year
Future	PM #5	10-year	10-year	<5-year	<5-year	<5-year
Conditions	PM #6	10-year	5-year	<5-year	<5-year	<5-year
SLR +3 feet	Overall LOS	<5-year	<5-year	<5-year	<5-year	<5-year









Maximum Stage and Bridge Low Chords – Snapper Creek Canal

						•												
				Current C	onditions			SL	R1			SL	R2			SL	R3	
	Elevation (ft-NAVD)																	
Location Description	Low Chord	Bridge Top	100 yr	25 yr	10 yr	5 yr	100 yr	25 yr	10 yr	5 yr	100 yr	25 yr	10 yr	5 yr	100 yr	25 yr	10 yr	5 yr
Turnpike entrance from SW 8th Street	12.16	16.18	5.65	5.08	4.69	4.37	6.06	5.31	5	4.73	6.46	5.81	5.32	5	6.81	6.25	5.87	5.6
Turnpike north bound exit to SW 9th Street	7.26	10.76	5.67	5.15	4.74	4.42	6.04	5.39	5.07	4.77	6.43	5.8	5.36	5.06	6.78	6.22	5.91	5.63
north bound entrance to Turnpike	9.57	13.93	5.78	5.19	4.75	4.44	6.02	5.48	5.09	4.77	6.41	5.8	5.39	5.1	6.75	6.21	5.9	5.64
SW 40th Avenue (Bird Drive)	5.57	7.46	5.78	5.19	4.75	4.44	6.02	5.48	5.09	4.78	6.41	5.8	5.38	5.09	6.75	6.21	5.9	5.64
SW 56th Avenue (Miller Drive)	8.37	12.06	5.82	5.2	4.73	4.41	6.08	5.5	5.1	4.75	6.38	5.82	5.41	5.1	6.7	6.21	5.89	5.63
SW 117th Ave	8.96	12.86	5.78	5.16	4.69	4.35	6.05	5.48	5.07	4.73	6.34	5.79	5.39	5.07	6.66	6.18	5.87	5.6
SW 107th Avenue	8.06	12.26	5.73	5.06	4.6	4.28	6.02	5.45	5.01	4.67	6.29	5.76	5.36	5.04	6.62	6.15	5.84	5.57
SW /2nd (Sunset)	7.08	10.13	5.68	5	4.53	4.21	5.98	5.4	4.96	4.62	6.24	5.72	5.32	5	6.58	6.12	5.81	5.54
SW 99th Avenue	8.06	10.21	5.67	4.97	4.49	4.15	5.98	5.39	4.94	4.6	6.23	5.72	5.32	4.99	6.57	6.11	5.81	5.53
R/R west of SR 874 Express Way	6.43	9.83	5.61	4.88	4.4	4.05	5.94	5.35	4.88	4.54	6.19	5.68	5.28	4.95	6.55	6.08	5.78	5.5
SR 874	10.9	13.9	5.58	4.85	4.36	4.02	5.92	5.32	4.85	4.51	6.17	5.67	5.26	4.93	6.53	6.07	5.77	5.49
SW87 Avenue	6.96	8.68	5.52	4.75	4.26	3.92	5.89	5.26	4.8	4.45	6.13	5.65	5.22	4.89	6.51	6.05	5.76	5.47
SW 79th (Kings Creek) Avenue	6.68	8.66	5.46	4.67	4.16	3.82	5.87	5.21	4.75	4.4	6.11	5.63	5.19	4.86	6.49	6.03	5.75	5.45
SW 77th Avenue	6.07	10.56	5.42	4.63	4.1	3.76	5.85	5.17	4.71	4.36	6.11	5.61	5.17	4.83	6.48	6.02	5.74	5.44
Palmetto Express Way + Ramp (combined)	7.1	13.76	5.38	4.58	4.04	3.7	5.83	5.14	4.68	4.32	6.18	5.59	5.15	4.81	6.65	6.07	5.73	5.49
Behind Dadeland Mall	7.46	8.11	5.38	4.57	4.03	3.69	5.82	5.13	4.67	4.32	6.17	5.59	5.15	4.81	6.65	6.06	5.73	5.5
SW 72nd Avenue	7.51	12.76	5.34	4.53	3.98	3.64	5.8	5.11	4.64	4.29	6.17	5.58	5.13	4.79	6.65	6.06	5.71	5.48
SW 70th Ave	8.29	13.6	5.3	4.48	3.93	3.59	5.78	5.07	4.61	4.26	6.15	5.56	5.11	4.77	6.64	6.05	5.7	5.46
US1 + Metro Rail (combined)	9.11	11.96	5.2	4.42	3.8	3.46	5.75	5.04	4.58	4.23	6.14	5.54	5.09	4.76	6.63	6.03	5.69	5.44
SW 67th (Ludlam) Avenue	8.78	13.08	5.19	4.29	3.78	3.45	5.72	5	4.55	4.19	6.12	5.53	5.06	4.74	6.62	6.02	5.67	5.42
SW 88th Street and SW 57th Avenue	4.16	8.89	5.03	4.17	3.61	3.3	5.67	4.92	4.46	4.09	6.09	5.48	5.01	4.69	6.59	6	5.65	5.41
				*High	lighted o	ells indi	cate the	stages e	xceed the	e bridge	low chord	ł						





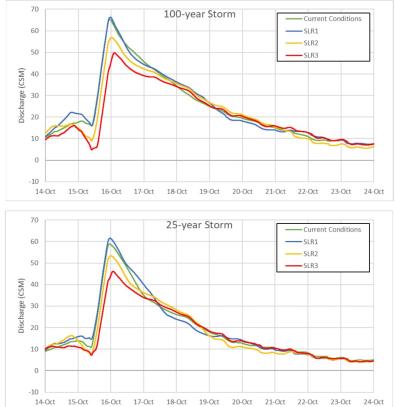
PM#2

Instantaneous Maximum Discharge Capacity – 100yr

	CURRENT CONDITIONS	SLR1	SLR2	SLR3					
Inflow Locations									
Start of Snapper Creek at C4 Canal (CFS)	-566.1	-775.9	-122.2	-66.1					
Future Connection at SW 157th Ave (CFS)	-	-403.9	-367.8	-178.7					
	Outflow Loc	ations							
Coral Way Canal at SW 117th Ave (CFS)	-87.4	-78.2	-18.7	-25.2					
SW 132nd Ave Canal into C4 Canal (CFS)	166.4	161.9	106.5	66.3					
\$112 (CFS)	0	0	0	0					
S22 Total Flow (CFS)	3163.5	2831.6	2734.5	2673.6					
	Watershed Su	Immary							
Basin Area (sq. mi.)		52.6	5						
Peak Watershed Discharge (CSM)	72.4	77.9	63	56.3					

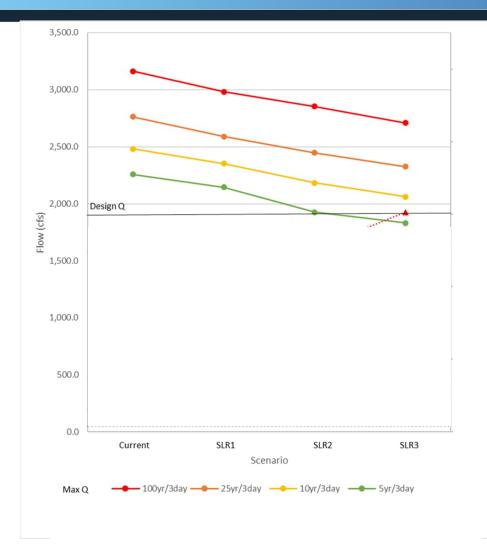
SLR1 discharge capacity increases above current conditions due to future planned canal connections to the C4 Watershed.

Maximum Discharge Capacity (12-Hour Moving Average)





PM#3

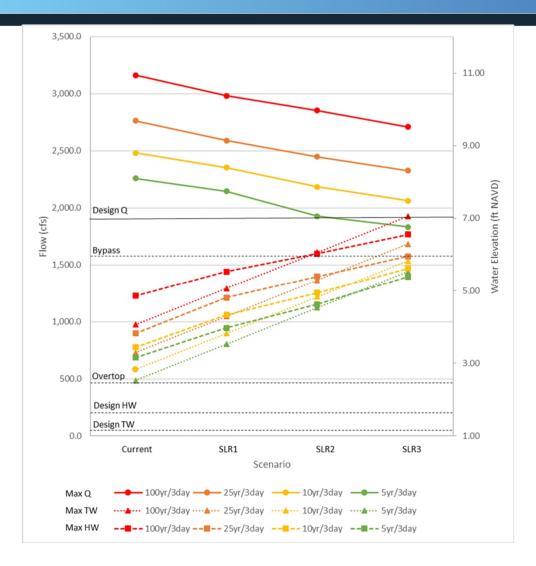


Structure Performance for S-22









Structure Performance for S-22

The increase in TW levels due to SLR, decreases the head differential at the structure and reduces flow.

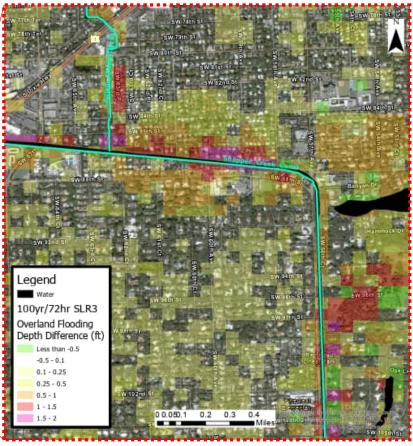


PM#5

• PM 5 – Maximum Flood Depth

Urban Flooding Depth Difference of SLR +3ft and Current Conditions for the 100-year Storm in the C2 Watershed

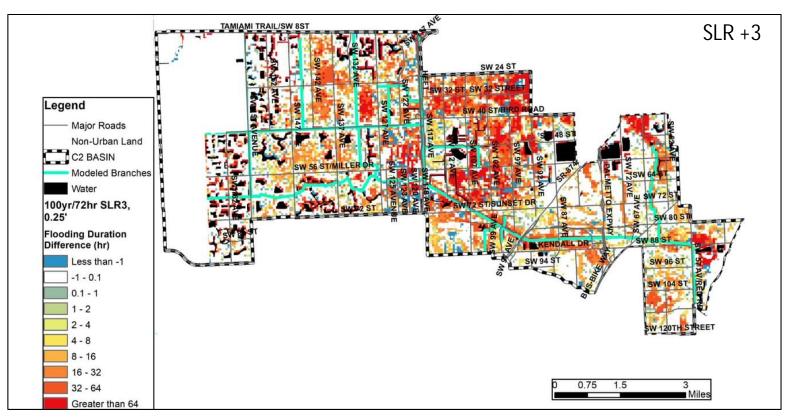








Maximum Flood Duration Difference between SLR and Current Conditions for the 100-year Storm





Results Summary – C2 Watershed

Increasing vulnerability of bridges. Future planned canals increase discharge to C4 Watershed. Extensive canal bank overtopping and overland flooding adjacent to the canals for all SLR conditions even at low storm frequencies. While flooding depth increases in areas adjacent to canals, flooding duration increases throughout the watershed.	METRIC	Notes	CURRENT CONDS.	SLR +1 FT	SLR +2 ft	SLR +3 FT
	PM #1	 SLR conditions increase the frequency of bridge low-chord exceedance for the SW 40th Ave and SW 5th Ave bridges. Additionally, the SW 77th Ave bridge low chord is exceeded with the SLR2 condition and the Railroad low chord west of SR874 is exceeded with the SLR3 condition for the 100-year storm. The number of culvert locations where the crown of road is exceeded increases significantly with each SLR condition. For the SLR1 5-year storm the same two (2) culverts are exceeded that were exceeded for the current conditions 10-year storm. The length of Snapper Creek that is overtopped is over a mile for all SLR conditions, with the exception of the SLR1 5-year 	10-year	5-year	<5- year	<5- year
	PM #2	 25-year Allowable Discharge exceeds the ERP value for the current and SLR1 conditions. With the future canals, a new connection to the C4 Canal increases discharges to the C4 watershed, increasing drainage capacity for SLR1 conditions. SLR2 conditions are similar to current conditions, and SLR3 reduces the discharge capacity at the S22 structure. 				
	PM #3	 Maximum discharge at S22 falls below design value for the 5-year event with SLR +3. The HW and TW exceed the water level that will bypass S22 for the 100-year design storm for SLR2 and SLR3 future scenarios and for the 25-year design storm for the SLR3 future scenario. 				
	PM #4	 Peak 12-hour moving discharge ranges from 1,725 CFS to 3,038 CFS (compared to the design discharge of 1,905 CFS) and decreases with increasing SLR for each design storm return period. 				
	PM #5	 17.6% of the watershed is flooded with 0.75 ft of depth or greater for the 100-year, 9.7% for the 25-year storm. Inundated areas at these depths are likely not a result of direct canal flooding, but due to slow drainage from flat areas far away from the canal system. Percent increases for the current conditions 25-year are comparable to the SLR1 numbers for the 25-year, while the percentages are similar to the 10-year storms for SLR2 and SLR3. 	25-year	25- year	10- year	10- year
	PM #6	 Canal: Stages at the T5W station recede after 68 hours for the 25-year storm for current conditions, and for more than 3 days for the 100-year storm, which is longer than the duration of the storm event itself. For SLR1, the canal recedes in less than 72 hours for the 10-year storm. For SLR2 and SLR3, the canal takes longer than 4 days to recede for all storm events. Watershed: Percent increases for the current conditions 25-year are comparable to the SLR1 numbers for the 25-year, while the percentages are similar to the 10-year storms for SLR2 and SLR3. 	25-year	25- year	10- year	10- year
		Overall Level of Service	10-year	5-year	<5- year	<5- year



<u>Western</u>

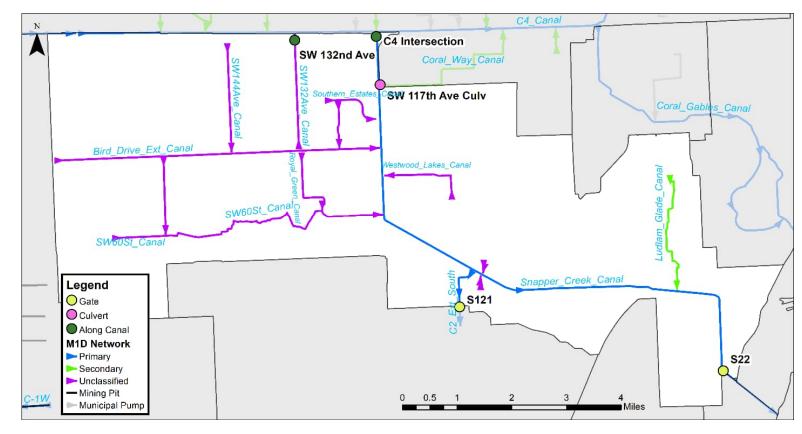
Acquire storage area

<u>Central</u>

- Raise embankments
- Temporary storage
- Municipal pump stations

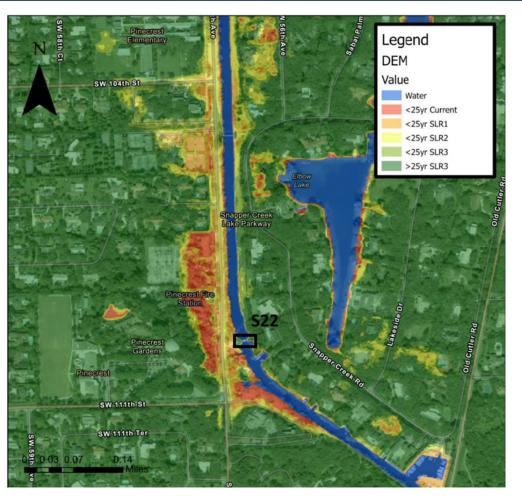
<u>Tidal</u>

- Structure hardening
- Forward pump station
- Canal Realignment









Tidal Improvements

- Increase existing structure elevation to prevent overtopping
- Add <u>tieback levees and floodwalls</u> to prevent short-circuiting around the structure (flanking)
- Install a <u>forward pump station</u> to help reach the design discharge under storm surge or SLR conditions
 - C2, C3W, C4, C5, C6 FPLOS for Current and Future SLR



Historic C2 Canal







Tidal Improvements





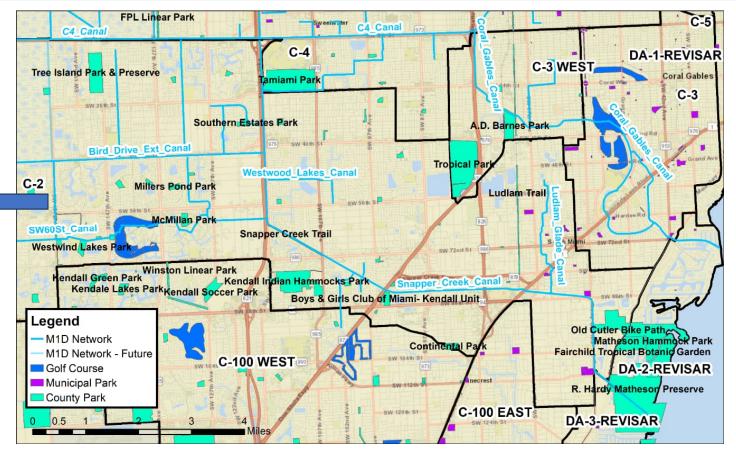
Central Improvements

- Raise canal embankments in problem areas
- Use Miami-Dade County parks or golf courses as emergency temporary storage
- Add municipal pumps to subbasins where gravity drainage will be affected by SLR



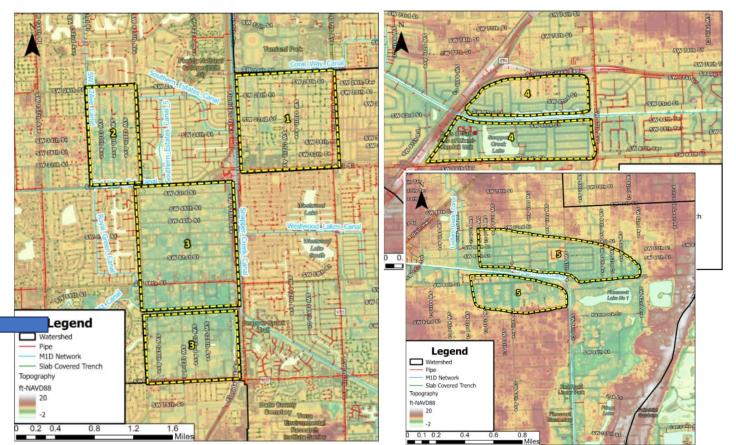


- **Central Improvements**
- Raise canal embankments in problem areas
- Use Miami-Dade County parks or golf courses as emergency temporary storage
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- **Central Improvements**
- Raise canal embankments in problem areas
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- Add municipal pumps to subbasins where gravity drainage will be affected by SLR





Western Improvements

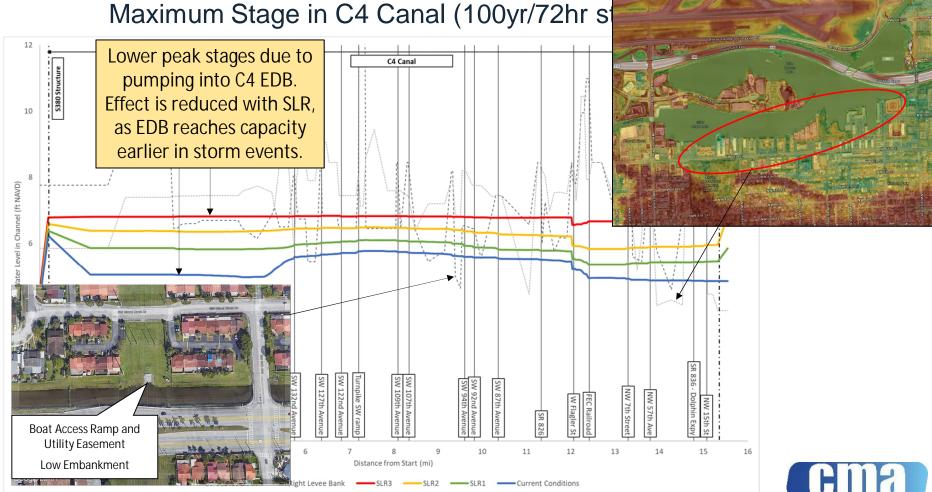
- Bird Drive Recharge Area could provide flood relief for multiple basins (C2, C3W, C4, and C5)
- Provide relief when C-4 Detention Basin reaches capacity
 - 100-year/72-hour for current conditions
 - 25-year/72-hour for SLR +1 ft





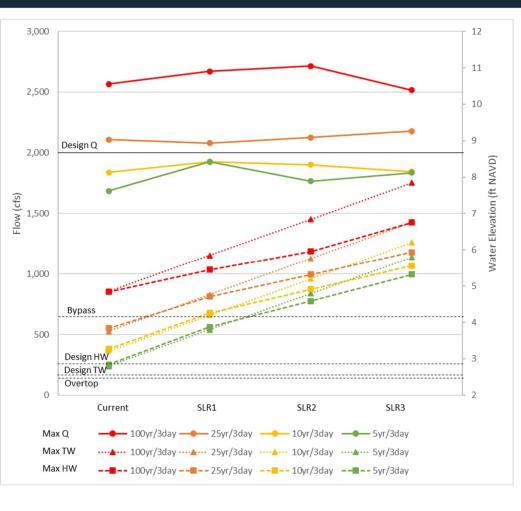


PM#1



chen moore and associates



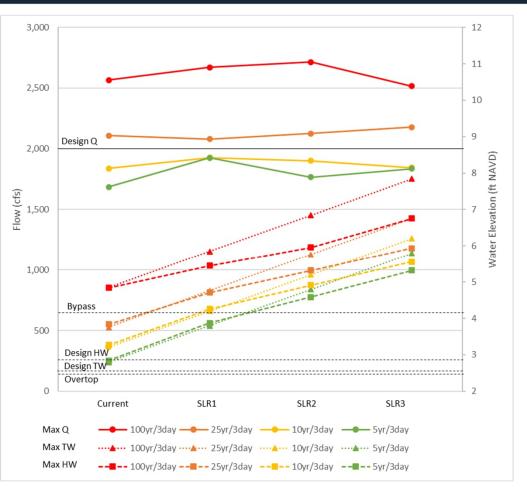


Structure Performance for S-25B







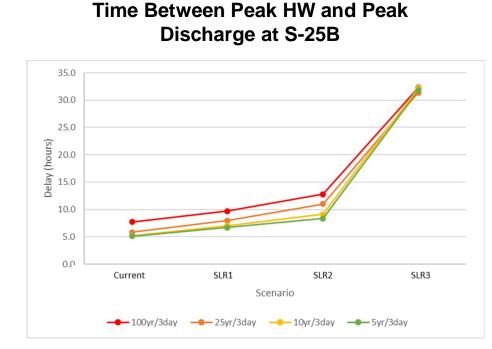


Structure Performance for S-25B

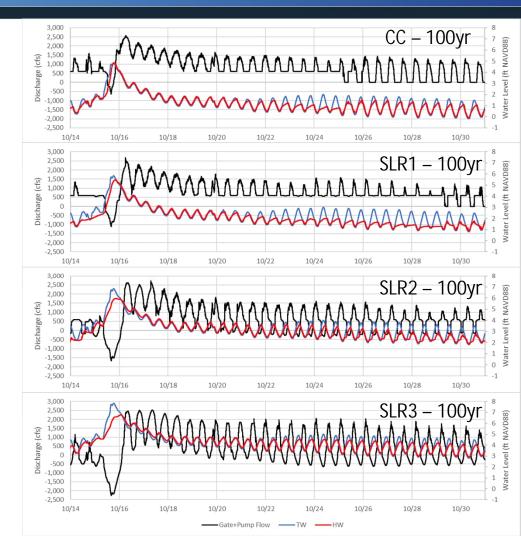
Peak discharge at S-25B does not seem to be affected by SLR.



PM#3

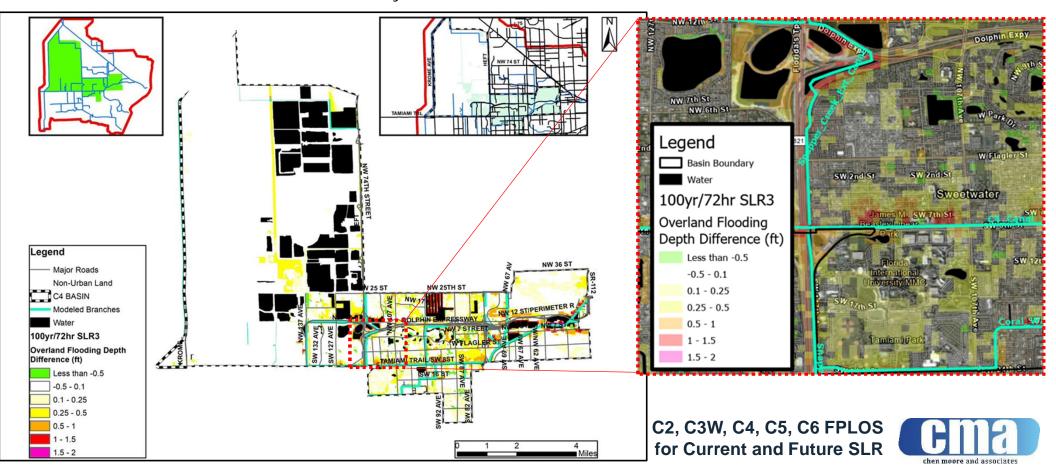


The max delay with respect to the peak discharge at S-25B increases by over two-fold from the current conditions and future SLR1 and SLR2 scenarios to the SLR3 scenario.



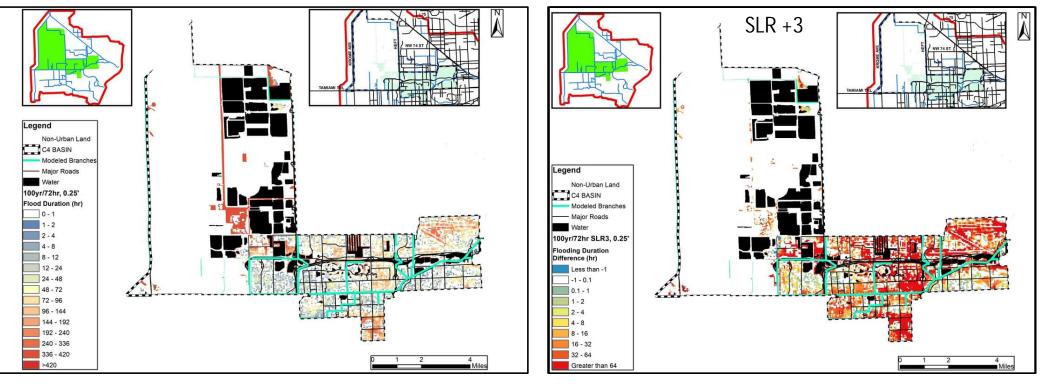


Urban Flooding Depth Difference of SLR +3ft and Current Conditions for the 100-year Storm



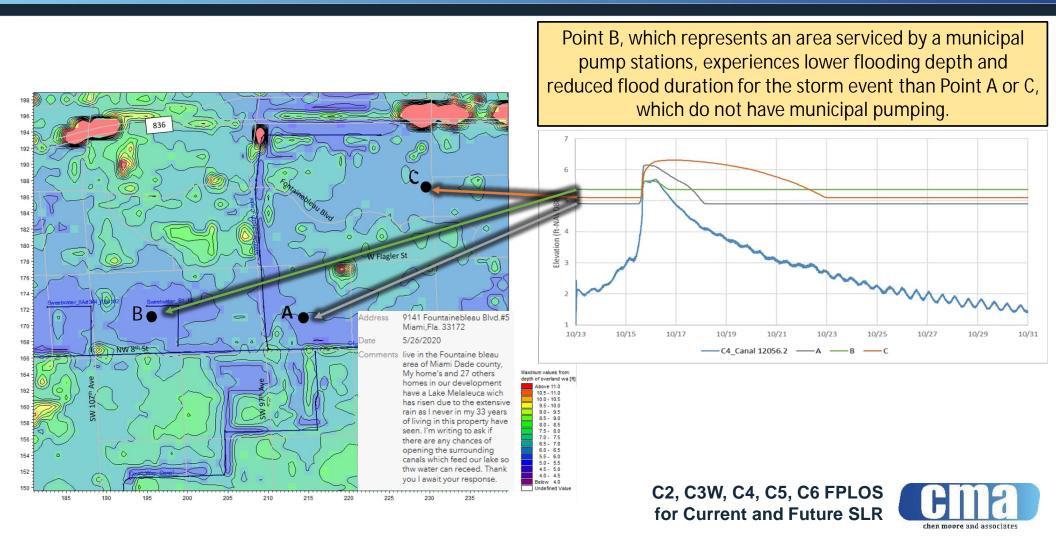


Urban Flood Duration Map for Current Conditions 100-year Storm Urban Flooding Duration Difference of SLR and Current Conditions for the 100-year Storm





PM#6



Results Summary – C4	Metric	Notes	CURRENT CONDS.	SLR +1 ft	SLR +2 ft	SLR +3 ft
Increasing canal bank overtopping with SLR scenarios.	PM #1	 No design storms exceeded bridge low chords in the C4 Canal. Five (5) of 61 culvert locations in the watershed were overtopped for the 25-year storm during current conditions, this doubled for SLR1. Three (3) culvert locations were overtopped for the 10-year storm for SLR1, but this increased to nine (9) for SLR2. Three (3) culvert locations were overtopped for the 5-year storm for SLR2, but this increased to 15 for SLR3. The C4 Canal is overtopped for less than a mile for the current conditions 25-year storm, for the SLR1 and SLR2 conditions for 10-year storm, and for the SLR3 condition for the 5-year storm. Problem areas include Fountainbleau, Westchester, and Doral East. 	25-year	10- year	5-year	< 5- year
Discharge capacity of the watershed is	PM #2	 No comparable value found for this basin. With the future canals, a new connection to the C4 Canal increases discharges from the C2 watershed, reducing drainage capacity for SLR1 conditions. Peak discharge capacity is delayed a full day for SLR2 and SLR3, when tailwater conditions have improved and pumping returns at S25B. 	-			
maintained with increasing SLR. However, discharge is delayed and extended in duration.	PM #3	 Maximum discharge at S25B falls below design value for the 5-year and 10-year storm for all current and future conditions. Max discharge is not affected by SLR. HIW exceeds the water level that will bypass S25B for the 100-year and 25-year current conditions scenarios and all future SLR scenarios (SLR1, SLR2, SLR3) except for the 5-year SLR1 scenario. The TW exceeds this bypass elevation during the 100-year current conditions scenario and all future SLR scenarios (SLR1, SLR2, SLR3) except for the 5-year SLR1 scenarios (SLR1, SLR2, SLR3) except for the 5-year SLR1 scenario. 	-	-		
	PM #4	 Peak 12-hour moving discharge ranges from 771 CFS to 2,159 CFS, compared to the design discharge of 2,000 CFS, and decreases with SLR. 				
Flooding depth and duration increases in areas adjacent to primary and secondary canals.	PM #5	 26.4% of the watershed is flooded with 0.75 in of depth or greater for the 100-year, 17.3% for the 25-year storm. Inundation at some of these locations is likely related to the canal stages overtopping the canal banks, however, some areas farther away from canals are also experiencing flooding at depths greater than 0.75ft. Difference maps suggest that OL flooding will increase primarily in areas adjacent to the C4 Canal such as Sweetwater and Fountainbleau. 	10-year	10- year	5-year	< 5- year
Municipal pumping appears to reduce flooding duration.	PM #6	 Canal: Stages at the T5W station recede after 68 hours for the 25-year storm for current conditions, and for more than 3 days for the 100-year storm, which is longer than the duration of the storm event itself. For SLR1, the canal recedes in less than 72 hours for the 10-year storm. For SLR2 and SLR3, the canal takes longer than 4 days to recede for all storm events. Watershed: Percent of area inundated for more than 48 hours for the current conditions 25-year is over 15% and this increases with SLR. 	10-year	10- year	5-year	< 5- year
		Overall Level of Service	10-year	10- year	5- year	<5- year

Western

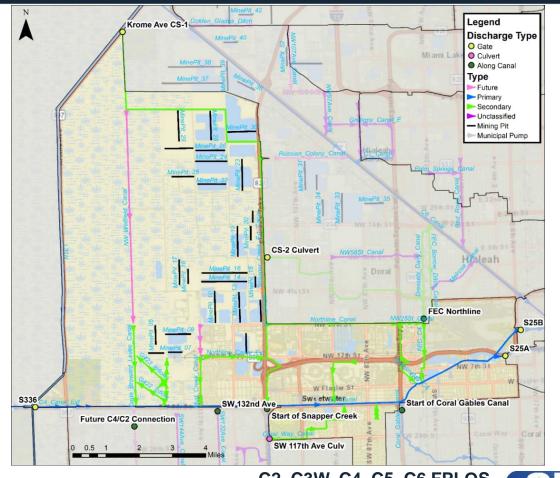
- Improvements to \$380
- C4 EDB expansion
- Acquire storage area

Central

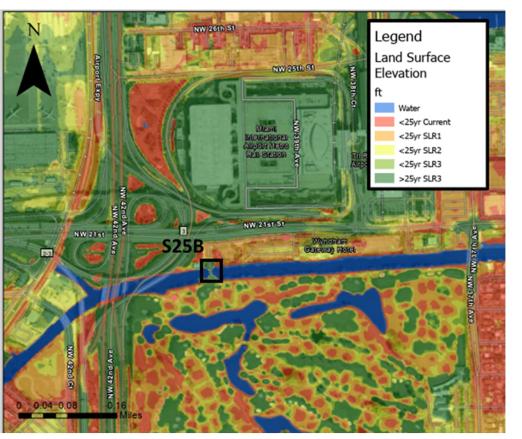
- Raise embankments
- Temporary storage
- Municipal pump stations
- Evaluate undersized culverts
- Additional control structures

<u>Tidal</u>

- Structure hardening
- Adjust Forward Pump Operations







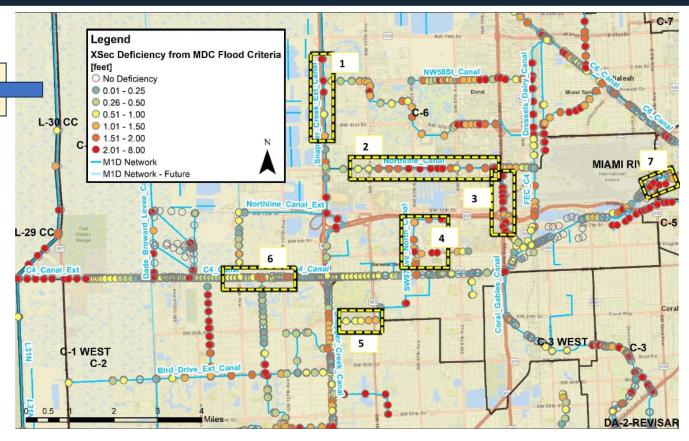
Tidal Improvements

- Increase existing structure elevation to prevent
 overtopping
- Add <u>tieback levees and floodwalls</u> to prevent short-circuiting around the structure (flanking)
- <u>Re-evaluate protocols at S25B_P</u> to allow for pumping during higher downstream conditions
- Pump capacity increases at S25B will be evaluated when downstream impacts are further analyzed in Phase 2



Central Improvements

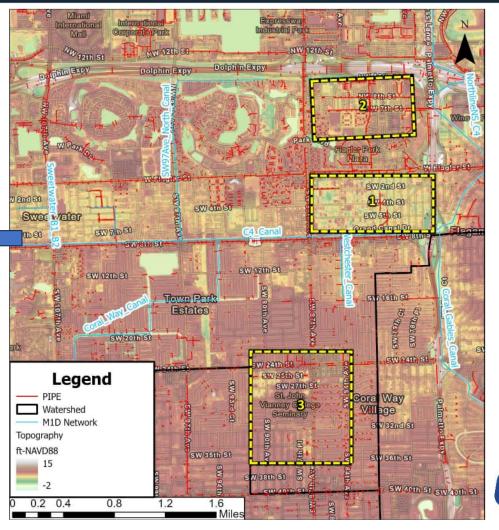
- Raise canal embankments in problem areas
- Use Miami-Dade County parks or golf courses as emergency temporary storage
- Add municipal pumps to subbasins where gravity drainage will be affected by SLR
- Evaluate undersized culverts
- Add control structures to connections with Snapper Creek Canal and Coral Gables Canal
- Separate C4 Canal from adjoining lakes to increase storage





Central Improvements

- Raise canal embankments in problem areas
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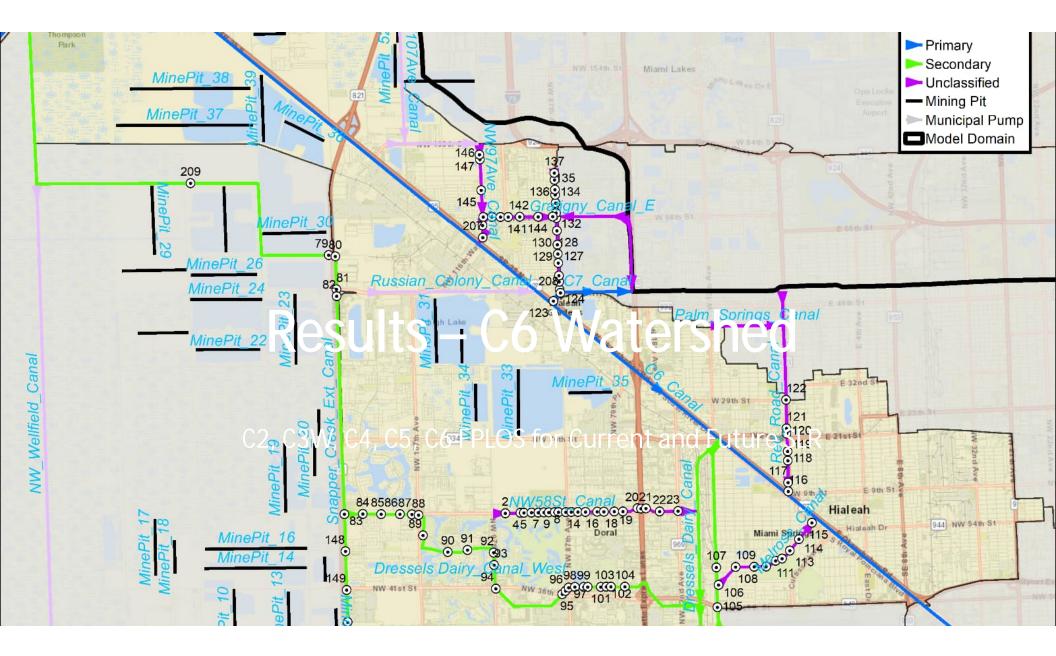


Western Improvements

- Structure upgrades to S380
- Expand C-4 Detention Basin
- Utilize Central Lake Belt Storage Area as additional storage during storm events





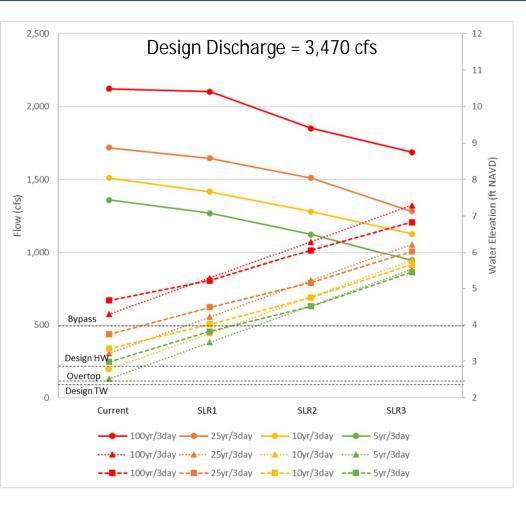




Maximum Stage in Miami Canal (100yr/72hr storm)





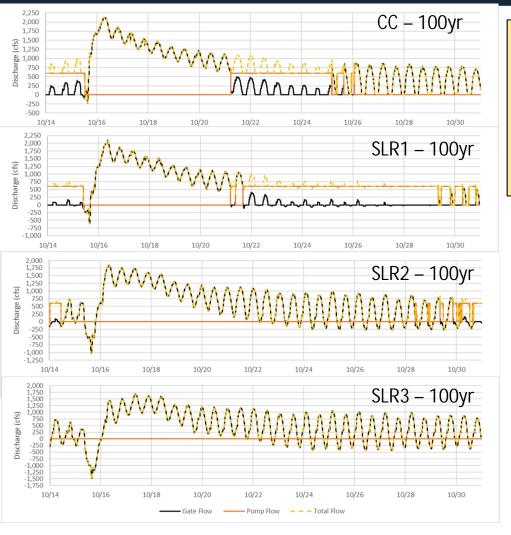


Structure Performance for S-26





PM#3

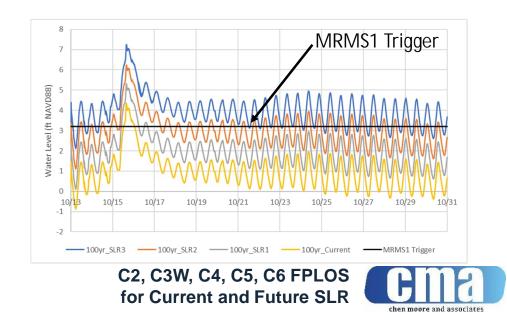


S-26 Pump does not turn on if the following conditions are met:

- Stage Upstream <-0.54 ft-NAVD
- Flow through S-26 gate
- S-25B Pump is off -

S-25B Pump does not turn on unless all three of the following conditions are met:

- Stage at T5W > 2.25 ft-NAVD
- Stage Upstream > -0.55 ft-NAVD
- Stage at MRMS1 <3.2 ft-NAVD</p>



PM#5

Urban Flooding Depth Difference of SLR +3ft and Current Conditions for the 100-year Storm

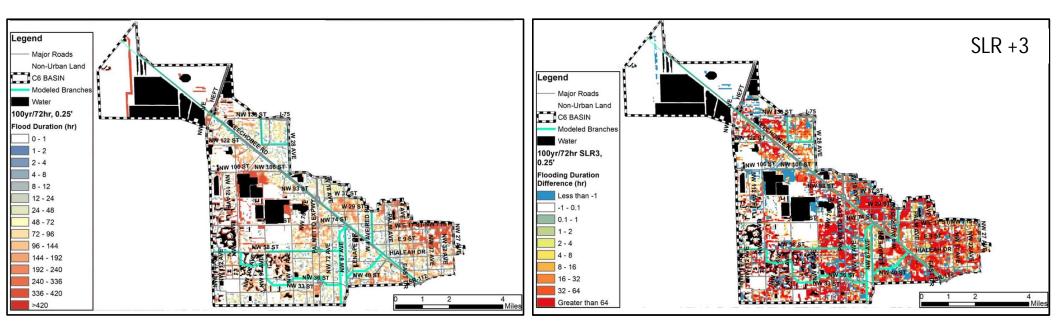


W 23rd St

W 21st s

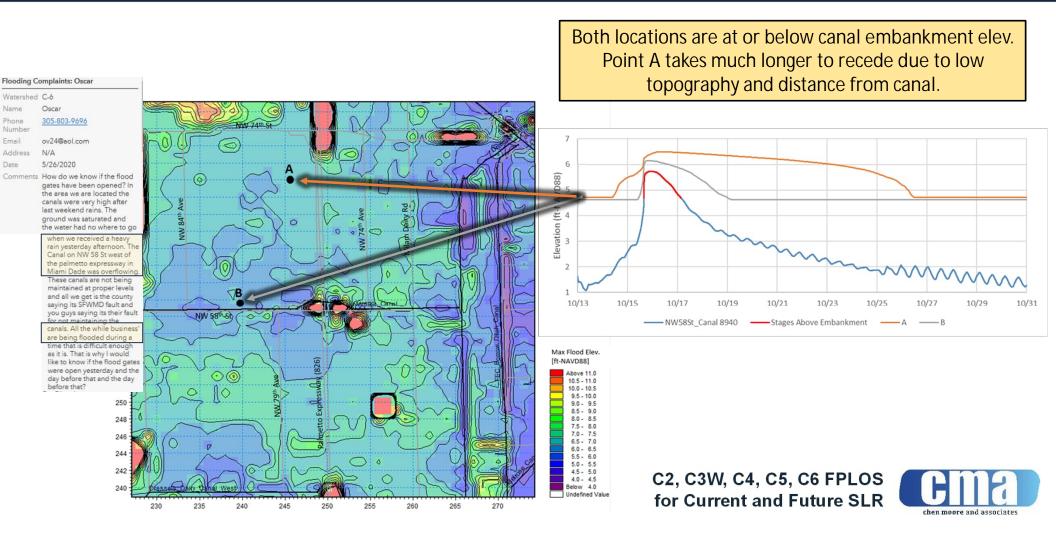


Urban Flood Duration Map for Current Conditions 100-year Storm Urban Flooding Duration Difference of SLR and Current Conditions for the 100-year Storm

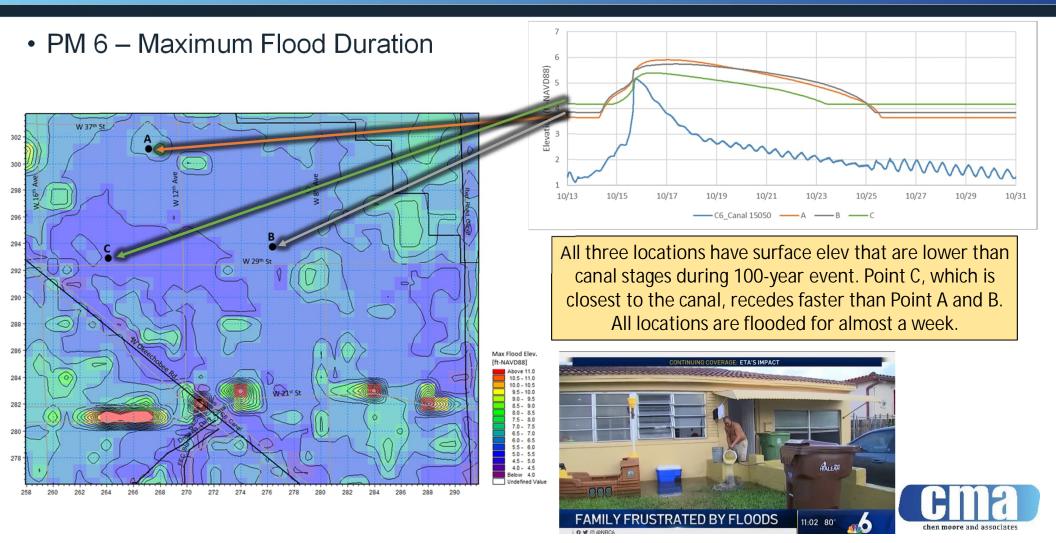








PM#6



Results Summary – C6 Watershed

Increasing canal bank	METRIC	Notes	CURRENT CONDS.	SLR +1ft	SLR +2ft	SLR +3ft
overtopping for some secondary canals and culvert locations. Pumping at S26_P helps maintain basin discharges for SLR1, but	PM #1	 Two bridge low chords are exceeded only for the SLR3 100-year simulation (Hook Square/Curtiss Pkwy & S Hook Square). For current conditions, 18 of 61 culverts experienced overtopping during the 100-year storm. Only two (2) experienced overtopping during the 25-year design storm. The number of culvert locations where the crown of road is exceeded increases significantly with each SLR condition, with the exception of the 5-year storm event that doesn't increase significantly until SLR3. 29% of the top-of-bank elevations along the C6 Canal were overtopped during the 100-year design storm, and only 5% during the 25-year. However, secondary canals such as NW 58th St Canal experience overtopping in some areas for all storm events, with majority of the canal passing the 25-year. Overtopping is comparable (and less than one mile) to the 25-year for current conditions for the SLR1 10-year. (See Table 9-83) 	25-year	10- year	< 5- year	< 5- year
pumping is reduced for SLR2 and SLR3 conditions due to	PM #2	 No comparable value found for this basin. There is a general decrease in discharge capacity for the watershed with each higher SLR condition due to reduced discharge at the S26 structure. However, SLR1 shows similar discharge capacity as the current conditions, due to increased pumping at S26_P 				
MRMS1 trigger. Some areas of Hialeah	PM #3	 Maximum discharge at S26 falls significantly below design value for all current and future conditions. HW exceeds the water level that will bypass S22 for 100-year current conditions and all future SLR scenarios except for the 5-year SLR1 scenario. The TW exceeds this bypass elevation during 100-year current conditions and all future SLR except for the 5-year and 10-year SLR1 scenarios. 				
and Doral show	PM #4	 Peak 12-hour moving discharge ranges from 664 CFS to 1903 CFS, compared to the design discharge of 3,470 CFS, and decreases with increasing SLR for each design storm return period. 				
and duration even during current conditions low frequency storm events. Multile flee align death	 The percentage of the watershed that is inundated with 3 inches or greater is still relatively high for the 5-year storm at 23.2% of the total urban area, however, this number drops rapidly to 15.0% for areas inundated with 6 inches or more. Areas such as Hialeah near W 12th Ave are low-lying and experience flooding due to peak stages in the C6 Canal for all storms except the 5-year for current conditions, however, inundation is comparable to the SLR1 5-year for higher depths. No other storms have comparable inundation. 	5-year	5- year	<5- year	< 5- year	
	 Canal: Using the new Reference Elevation, stages at the S26_H recede during the current conditions, SLR1, and SLR2 in less than 72 hours for all storm events. For SLR 3 this lasts for a week or more. Watershed: Portions of the watershed (such as Hialeah) show high flooding duration (i.e., > 72 hours) for all storm events. 	<5-year	<5- year	<5- year	< 5- year	
increases in areas adjacent to the canal,	1	Overall Level of Service	5-year	5- year	<5- year	<5- year
flooding duration increases throughout the watershed.		C2, C3W, C4, C5, C6 FP for Current and Future		C	ÎF	

chen moore and associates

Western

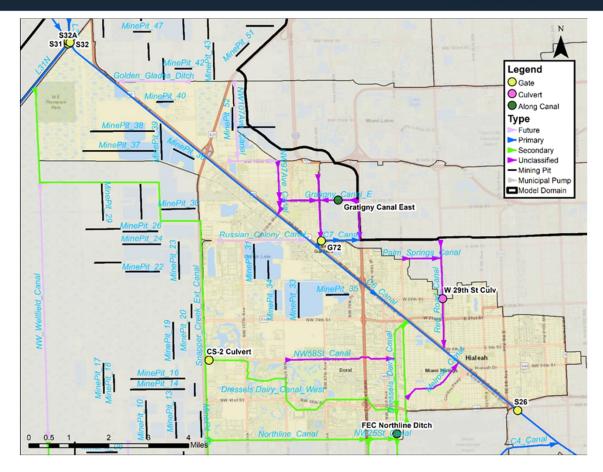
• Acquire storage area

Central

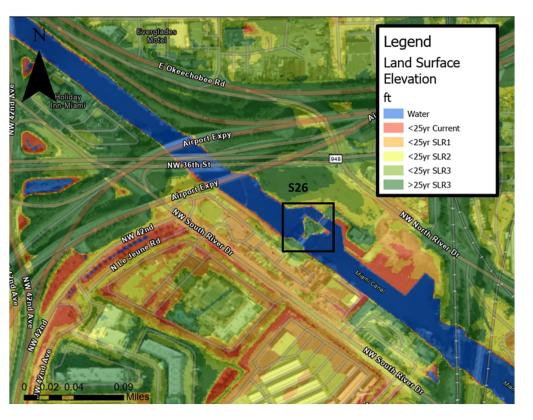
- Raise embankments
- Temporary storage
- Municipal pump stations
- Widen canal
- Undersized culverts
- G72 improvements

<u>Tidal</u>

- Structure hardening
- Adjust Forward Pump Operations
- Additional structure at mouth of canal







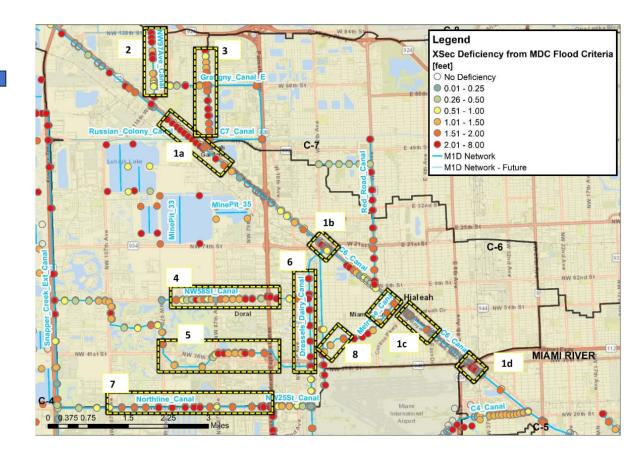
Tidal Improvements

- Increase existing structure elevation to prevent overtopping
- Add <u>tieback levees and floodwalls</u> to prevent flanking
- <u>Re-evaluate protocols at S25B_P</u> to allow for pumping during higher downstream conditions
- Retrofit existing forward pumps to increase pump capacity and operability. Downstream impacts will be evaluated in Phase 2.
- <u>Sector gate and pump</u> at mouth of Miami River (recommended in Back Bay Study)



Central Improvements

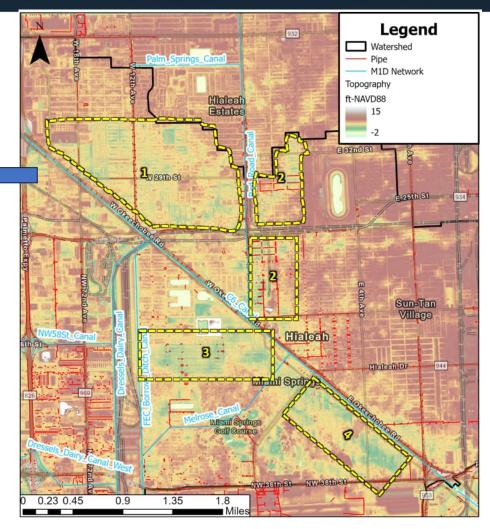
- Raise canal embankments in problem areas
- Add municipal pumps to subbasins where gravity drainage will be affected by SLR
- Use Miami-Dade County parks or golf courses as emergency temporary storage
- Widen C6 canal to be able to meet design discharge at S26
- Evaluating undersized culverts
- G72 improvements and connection to C7 Canal





Central Improvements

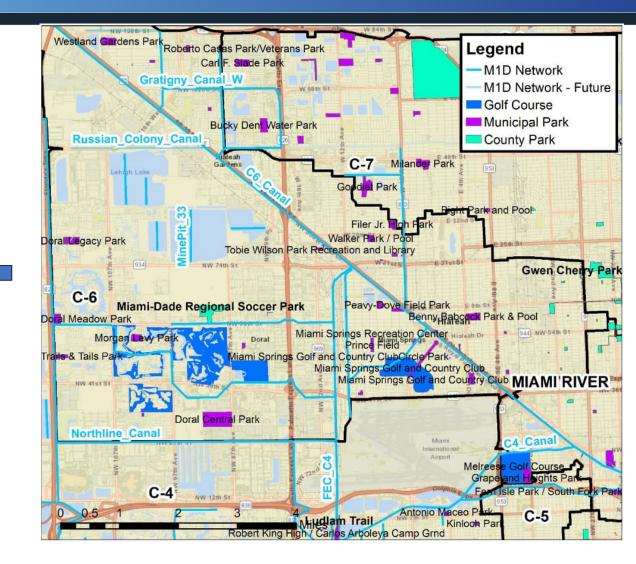
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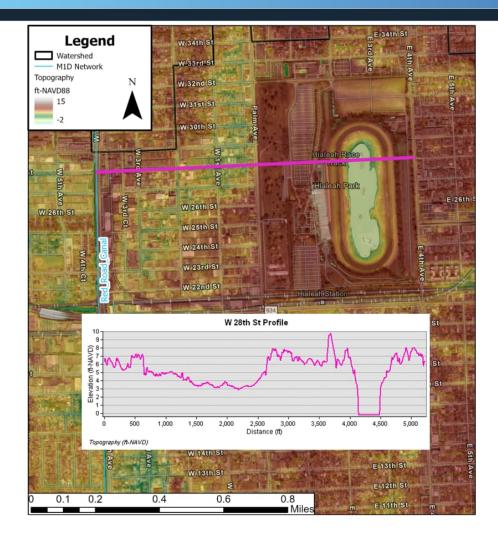




Central Improvements

- Raise canal embankments in problem areas
- Add municipal pumps to subbasins where gravity drainage will be affected by SLR
- Use Miami-Dade County parks or golf courses as emergency temporary storage
- Widen C6 canal to be able to meet design discharge at S26
- Evaluating undersized culverts
- G72 improvements and connection to C7 Canal





Central Improvements

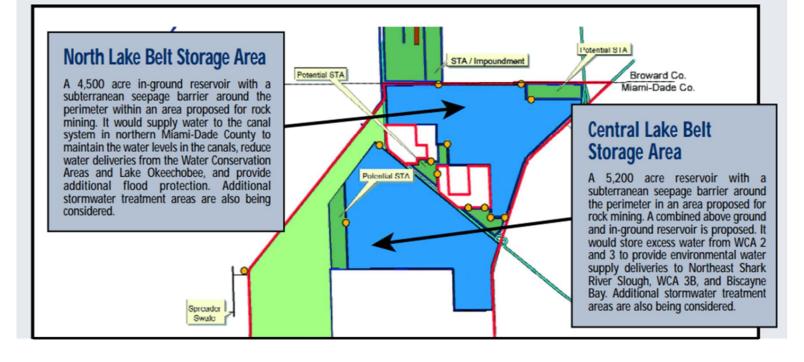
- Use Miami-Dade County parks or golf courses as emergency temporary storage
- Hialeah Park Casino
 - Provide controlled storage using naturally high landscape as levees
 - Degrade the natural areas to provide additional floodplain storage during high rainfall events



Western Improvements

 Utilize North Lake Belt Storage Area as additional storage during storm events

CERP COMPONENTS MAP





Future Studies

- SFWMD's Resiliency Plan
 - Develop Conceptual Costs for Mitigation Strategies
- US Army Corps of Engineers
 - 216 Study
- Phase 2
 - Model select mitigation strategies
 - Evaluate downstream impacts of adding or increasing pump stations at coastal structures
 - Work with stakeholders



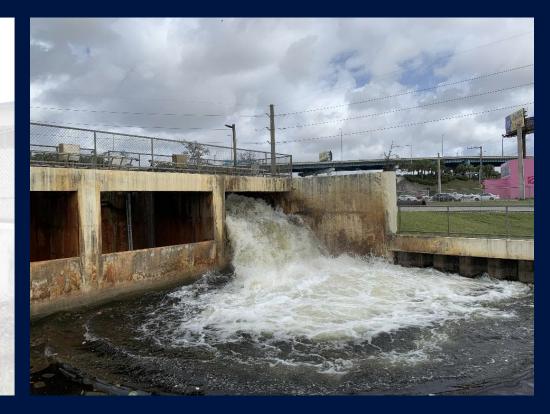


Thank You!

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Preliminary Mitigation Strategies - Summary

C-9 WE ST BASIN NORTH BISCAYNE BAY BASIN	BASIN	8 tidal structure improvement/ mitigation projects	20 mitigation projects 13 District owned/ co-owned solutions	6 basin headwater mitigation projects
C2 BASIN C7 BASIN C7 BASIN NORTH BISCAYNE BAY BASIN	C2	 S22 hardening (Raise the overtopping and bypass elevations, add tie-back levees/floodwalls) Forward pump station Canal re-alignment 	 Raising canal embankments in problem areas Temporary storage in parks/golf courses Municipal pump station (including pumping from Ludlam Glade contributing basin up to Coral Gables Canal) Extend SW 157th Avenue Canal to the C4 Canal Sub-dividing the C2 Watershed to increase discharge potential 	1. Acquire storage in western mining lakes with water control structures in Bird Drive Extension Canal to convey water to storage facilities
COBASIN MIAMI RIVER BASIN COBASIN	сзw	 G93 hardening (Raise the overtopping elevation) Additional salinity structure or storm surge/tidal barrier at the end of the C3 Canal (potentially with navigational accessibility) 	 Raising canal embankments in problem areas Temporary storage in parks/golf courses 	
C2 BASIN C2 BASIN	C4	 S25B Structure Upgrades – raising the overtopping elevation, adding tie-back levees Adjust forward pump operations to allow for discharge under higher downstream conditions 	 Raising canal embankments in problem areas Temporary storage in parks/golf courses Municipal pump station improvements Evaluating undersized culverts Control structures at C2 and C3W watersheds Separating storage areas from main canal 	 Improved operations for S380 to keep water west C4 Emergency Detention Basin Expansion Acquire storage areas in western mining lakes (Central Lake Belt Storage Area)
C-100 WESTEASIN C-100 WESTEASIN C-100 EASTEASIN C-100 EASTEASIN	C5	 S25 replacement (construct spillway in same location with tie-back levees/floodwalls) S25 relocation (construct spillway in location with higher elevation and with tie-back levees/floodwalls) Forward pump station at S25 	 Raising canal embankments in problem areas Municipal pump stations 	1. Improvements to S25A to allow inter-basin connection with C4 Canal
2 4 8 Miles Corris Bold, Marcel, & vsBys, Earlingtor & Model, DoMain Basins in Study Miles AuseRid, UBes, AveeRid, 1941, and 80, etc. AdJACENT BASINS	C6	 S26 Hardening (Raise the overtopping and bypass elevations, add tie-back levees/floodwalls) Adjust forward pump operations for SLR scenarios Potential retrofit of existing forward pump stations Floodwalls, sector gate, and pump station at the mouth of Miami River 	 Raising canal embankments in problem areas Widen canal to improve conveyance capacity Construct municipal pumps for Hialeah and Doral Temporary Storage in parks/golf courses Improvements and operational changes to G72 to discharge to C7 Canal during storm events 	1. Acquire storage areas in western mining lakes (North Lake Belt Storage Area) with conveyance structures connecting to C6 Canal